Proposed Temporary Open Storage of Construction Materials for a Period of 3 Years and Associated Filling of Land in "Agriculture" Zone, Lots 1290 RP (Part) and 1291 (Part) in D.D. 107, Fung Kat Heung, Kam Tin, Yuen Long, New Territories

Drainage Appraisal

<mark>April 2024</mark>

Table of Content

1.	Intr	oduction	1
	1.1	Background	1
	1.2	The Site	1
2.	Dev	elopment Proposal	2
	2.1	The Proposed Development	2
3.	Asse	essment Criteria	2
4.	Pro	posed Drainage System	5
5.	Con	clusion	5

List of Table

Table 1 - Key Development Parameters	2
Table 2– Design Return Periods under SDM	2

List of Figure

- Figure 1 Site Location Plan
- Figure 2 Existing Drainage Plan
- Figure 3 Proposed Drainage System
- Figure 4 Catchment Plan

List of Appendix

- Appendix A Design Calculation
- Appendix B Development Layout Plan
- Appendix C Reference Drawings for Uchannel and Catchpit
- Appendix D Photo Record of Surroundings
- Appendix E Sections
- Appendix F Checking of Existing 3m (W) x 2m (D) Channel

1. Introduction

1.1 Background

- 1.1.1 The applicant seeks planning permission from the Town Planning Board (the Board) to use Lots 1290 RP (Part) and 1291 (Part), Fung Kat Heung, Kam Tin, Yuen Long, New Territories (the Site) for 'Proposed Temporary Open Storage of Construction Materials for a Period of 3 Years and Associated Filling of Land' (proposed development).
- 1.1.2 This Drainage Proposal is to support the planning application for the proposed use.

1.2 The Site

- 1.2.1 The Application Site area is about 3,796m², and it is at rural area of Kam Tin North. The site is currently an open area with existing temporary structures. The site location plan is shown in **Figure 1**.
- 1.2.2 The Application Site is surrounded by temporary structures and local track. The existing ground level various from approx. +10.3 mPD to +11.7 mPD. The site is intended to be filled and paved to ground level from +11.2 mPD to +11.9 mPD.
- 1.2.3 There is an existing channel adjacent to the site at the west and south. Existing Drainage Plan is shown in **Figure 2** for reference.
- 1.2.4 The site at the east is slightly higher than the Application Site. The eastern area is mostly grassland. It is assumed that the unpaved area is 75% for assessment purposes. Catchment Plan is shown in Figure 4.
- 1.2.5 Proposed Development Layout plan is shown in **Appendix B** for reference.

Proposed Temporary Open Storage of Construction Materials for a Period of 3 Years and Associated Filling of Land in "Agriculture" Zone, Lots 1290 RP (Part) and 1291 (Part) in D.D. 107, Fung Kat Heung, Kam Tin, Yuen Long, New Territories

2. Development Proposal

2.1 The Proposed Development

2.1.1 The total site area is approximately 3,796m². The indicative development schedule is summarized in Table 1 below for technical assessment purpose. The proposed catchment plan is shown in **Figure 4**.

Proposed Development				
Total Site Area (m ²)	3,796			
Assume all proposed site area as paved				
area after development for assessment				
purpose				
Zone A (m ²)	<mark>1,700</mark>			
Zone B (m ²)	<mark>2,096</mark>			
External Catchment				
Eastern External Catchment (m ²)	<mark>4,246</mark>			
The Area is mostly grassland				
It is assumed that paved to unpaved				
ratio is 20:80 for assessment purposes.				

 Table 1 - Key Development Parameters

3. Assessment Criteria

3.1.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this DIA. The recommendation is summarized in **Table 2** below.

Description	Design Return Periods
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage Including Internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

Table 2– Design Return Periods under SDM

3.1.2 The proposed village drainage system intended to collect runoff from the internal site and discharge to existing nearby existing stream. 1 in 10 years return period is adopted for the drainage design.

- 3.1.3 Stormwater drainage design will be carried out in accordance with the criteria set out in the Stormwater Drainage Manual published by DSD. The proposed design criteria to be adopted for design of this stormwater drainage system and factors which have been considered are summarised below.
 - 1. Intensity-Duration-Frequency Relationship The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the HKO Headquarters Rainfall Zone. Therefore, for 10 years return period, the following values are adopted.

а	=	471.9
b	=	3.02
с	=	0.397

2. The peak runoff is calculated by the Rational Method i.e. $Q_p = 0.278 \text{CiA}$

where	Q_p	=	peak runoff in m ³ /s
	С	=	runoff coefficient (dimensionless)
	i	=	rainfall intensity in mm/hr
	А	=	catchment area in km ²

- 3. The run-off coefficient (C) of surface runoff are taken as follows:
 - Paved Area: C = 0.95
 Unpaved Area: C = 0.35

4. Manning's Equation is used for calculation of velocity of flow inside the channels:

Manning's Equation: $v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$

Where,

V = velocity of the pipe flow (m/s)S_f = hydraulic gradient n = manning's coefficient R = hydraulic radius (m)

5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes:

 $\underline{v} = -\sqrt{32gRS} \log \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}}\right)$ Colebrook-White Equation: where, V velocity of the pipe flow (m/s) = Sf = hydraulic gradient k_f roughness value (m) = kinematics viscosity of fluid v = D pipe diameter (m) = R = hydraulic radius (m)

4. Proposed Drainage System

- 4.1.1 The Application Site and the surrounding areas are generally flat with gentle gradient toward the west. Drainage system is proposed to collect the runoff from the application site and external catchment at the east. It is proposed to discharge to existing 3m (W) x 2m (D) channel at the west. The alignment, size and gradient of the proposed drains are shown in **Figure 3**.
- 4.1.2 The design calculations of proposed channels are shown in **Appendix A**.
- 4.1.3 The design checking of existing 3m (W) x 2m (D) channel at the west for drainage discharge is shown in Appendix F.
- 4.1.4 The reference drawings of proposed drains are shown in **Appendix C**.

5. Conclusion

- 5.1.1 A drainage appraisal has been conducted for the Proposed Development. The surface runoff from the Application Site will be collected by the proposed perimeter Uchannel/drains and discharged to the existing stream at the west.
- 5.1.2 With the proposed drainage system, it is anticipated that there will be no significant drainage impact to the area after the implementation of the development.

End of Text -

FIGURES



	PROJECT: Proposed Temporary Open Storage of Construction Materials for a Period of 3 Years and Associated Filling of Land in "Agriculture" Zone, Lots 1290 RP (Part) and 1291 (Part) in D.D. 107, Fung Kat Heung, Kam Tin, Yuen Long, New Territories
	REV DESCRIPTION DATE DRAWING TITLE SITE LOCATION PLAN
ONLY)	DRAWING NUMBER FIGURE 1



LEGEND:

	Combined Manhole
ъ	Overflow (Combined)
—	Pipe (Combined)
	Interface Valve Chamber
	Sewer Manhole
-	Oil / Petrol Interceptor
S	Overflow (Sewer)
_	Pipe (Sewer)

н	Tapping Point (Sewer)	н	Tapping Poir
	Sewer Terminal Manhole	٥	Storm Wate
•	Catchpit	7222	Tunnel Prote
↦	Inlet	<i>7222</i>	Tunnel Prote
0	Storm Water Manhole		Tunnel / Box
+(Outlet	8008	Tunnel / Box
-	Pipe (Storm)		
-	Sand Trap		

н	Tapping Point (Storm)		
•	Storm Water Terminal Manhole		
7223	Tunnel Protection Zone (100m / 200m)		
7 <i>223</i>	Tunnel Protection Zone (General Range)		
	Tunnel / Box Culvert (Sewer)		
111	Tunnel / Box Culvert (Storm)		

med Forces ound Training Center	PROJECT: Proposed Temporary Open Storage of Construction Materials for a Period of 3 Years and Associated Filling of Land in "Agriculture" Zone, Lots 1290 RP (Part) and 1291 (Part) in D.D. 107, Fung Kat Heung, Kam Tin, Yuen Long, New Territories		
E DN ONLY)			
1 1 1			
	REV	DESCRIPTION	DATE
	DRAWIN EXIST PLAN	IG TITLE TING DRAINAG	θE
N MAR 2024	drawin FIGUF	IG NUMBER RE 2A	



IN 200 IN 200 IN 200				
	PROJ Propos Storage Materia Years a of Land Lots 12 (Part) i Heung New Te	ECT: ed Tempo e of Const als for a P and Assoc f in "Agric 290 RP (P n D.D. 10 , Kam Tin, erritories	orary Op truction eriod of ciated Fi ulture" Z art) and 7, Fung , Yuen L	oen 3 Iling Zone, 1291 Kat .ong,
R				
15				
12.8				
TS R				
2				
	REV	DESCRIP	TION	DATE
	PROP	OSED D EM	RAINA	\GE
POINT SHOULD				
ONSTRUCTION. ONLY WHICH		IG NUMBEI RE 3A	2	



Appendix

endix A - Desigr	n Calculation			
U Channel 1 - Zone A with	External Catchment C1			
Runoff Estimation				1
Design Return Period		1 in 10	vears	
Paved Area	$1700 + 4246 \times 0.3 =$	2974	(m2)	
I Innaved Area	4246 x 0.7 -	2074	(m2)	
Total Equivalent Area	$-270 \times 0.7 =$ 2074 × 0.05 ± 2072 × 0.35 =	3865	(m2)	
Time of Concentration *	2314 x 0.33 + 2372 x 0.33 -	6 20	(IIIZ)	
Poinfall Intensity 1 **		105	mm/hr	$\star i = \frac{u}{1}$
Design Discharge Rote	0.278 x 2865 x 105 / 1000000 -	0.010	m2/o	$(t_d + b)^c$
Design Discharge Rate, Q	0.276 X 3005 X 1957 1000000 =	0.210	1113/5]
11 Channel				1
Channel Size		525	(mm)	
Gradiont		1 in 200	(11111)	
Velocity		162	m/c	
Capacity		0.400	m2/c	
Capacity		0.400	1113/5	1
Utilization	0.21 / 0.4	= 52.54	%	OK (Allowed 10% for siltation)
II Channel 2 - Zone B				
Runoff Estimation				1
Design Return Period		1 in 10	Voore	4
Design Return Fellou	2000	100	years (m 0)	
Paved Area	2096	2096	(m2)	
Unpaved Area		0	(m2)	
Total Equivalent Area	$2096 \times 0.95 + 0 \times 0.35 =$	1991	(m2)	
Time of Concentration *		6.20	min	а
Rainfall Intensity, I **		195	mm/hr	$i = \frac{1}{(1-1)}$
Design Discharge Rate, Q	0.278 x 0 x 195 / 1000000 =	0.108	m3/s	$(l_d + b)^c$
				7
U Channel		075	()	
Channel Size		3/5	(mm)	
Gradient		1 in 200		
Velocity		1.30	m/s	
Capacity		0.163	m3/s	1
Utilization	0.108 / 0.163	= 66.39	%	OK (Allowed 10% for siltation)
U Channel 3 - Combined (2	Zone A + B +C1)			
Runoff Estimation				
Design Return Period		1 in 10	years	
Paved Area	2974 + 2096 =	5070	(m2)	
Unpaved Area	2972 + 0 =	2972	(m2)	
Total Equivalent Area	5070 x 0.95 + 2972 x 0.35 =	5857	(m2)	
Time of Concentration *		6 20	min	
Rainfall Intensity 1 **		105	mm/br	$** i = \frac{u}{1}$
Design Discharge Rate O	0 278 x 5857 x 195 / 1000000 -	195 0 210	m2/e	$(t_d + b)^c$
Design Discharge Rate, Q	0.270 × 3037 × 1337 1000000 =	0.510	110/3	
U Channel				1
Channel Size		600	(mm)	1
Gradient		1 in 200	()	
Velocity		1 78	m/s	
Capacity		0.570	m3/s	
Utilization		EE 70	0/	
Utilization	0.318 / 0.57	= 55.76	%	OK (Allowed 10% for siltation)

Time of Concentration for External Catchment C1

Catchment	Flow Distance	Highest	Lowest	Gradient (per 100m)	to (min) =	tc =
		Level	Level	= (H1-H2)/L x 100	0.14465L/ (H ^{0.2} A ^{0.1})	to + tf
А	L			Н		
(m2)	(m)	(mPD)	(mPD)		(min)	(min)
4246	166	34.5	12.5	13.253	6.211	6.211
tc	=	6.211	(min)			



Appendix C - Reference Drawings





ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- 11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- 12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

	A	MINOR AMENDMENT.	Original Signed 04.2016	
	REF.	FORMER DRG. NO. C2406J. REVISION	Original Signed 03.2015 SIGNATURE DATE	
CATCHPIT WITH TRAP	CI.	CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT		
(SHEET 2 OF 2)	SCAL DATE	E 1:20 JAN 1991	drawing no. C2406 /2A	
卓越工程 建設香港	卓越工程 建設香港 We Engineer Hong Kong's Development			















Time of Concentration for Existing 3m (W) x 2m (D) Channel Highest Gradient (per 100m) to (min) = tc = Lowest Catchment Flow Distance 0.14465L/ (H^{0.2}A^{0.1}) Level Level = (H1-H2)/L x 100 to + tf Н А L (m2) (m) (mPD) (mPD) (min) (min) 530101 1446 374 10.2 25.159 29.371 29.371

